

#### IN ACCORDANCE WITH:

THE 2017 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS 8TH EDITION FOR HL-93 LOADING.

THE 2013 MASSACHUSETTS DEPARTMENT OF TRANSPORTATION LRFD BRIDGE MANUAL.

THE 2011 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS GUIDE SPECIFICATIONS FOR LRFD SEISMIC BRIDGE DESIGN WITH 2012 INTERIM REVISIONS.

#### BENCH MARKS:

1. SPIKE IN U.P. 1 N3042949.207, E315500.777, EL. 856.69 2. SPIKE IN U.P. 2 N3042785.145, E315440.490, EL. 854.10

ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD88) OF 1988.

#### DATE:

TO BE PLACED ON THE FACE OF THE SOUTHEAST AND NORTHWEST WINGWALLS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST WINGWALL IS CONSTRUCTED. BOTH WINGWALLS SHALL FEATURE THE SAME DATE.

#### SURVEY NOTEBOOK:

SURVEY PERFORMED BY SHERMAN & FRYDRYK, LLC.

#### UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

SEISMIC GROUND SHAKING HAZARD:

#### DESIGN SPECTRA:

As = 0.072g Sds = 0.156gSd1 = 0.068g

SITE CLASS = C

SEISMIC DESIGN CATEGORY (SDC) = A

#### **GEOTECHNICAL REPORT:**

REFER TO GEOTECHNICAL REPORT, DATED JANUARY 2019, PREPARED BY GEODESIGN INCORPORATED.

#### **REINFORCEMENT:**

REINFORCING STEEL SHALL BE EPOXY COATED UNLESS OTHERWISE NOTED IN THE CONSTRUCTION DRAWINGS AND SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M31 GRADE 60. ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS
1. NONE	21"	26"
2. 12" OF CONCRETE BELOW BAR	29"	36"
3. COATED BARS, COVER <3dь, OR CLEAR SPACING <6dь	31"	39"
4. COATED BARS, ALL OTHER CASES	25"	31"
5. CONDITION 2, AND 3	35"	44"
6. CONDITION 2, AND 4	34"	43"

IF THE ABOVE BARS ARE SPACED 6 INCHES OR MORE ON CENTER, THE LAP LENGTH SHALL BE 80% OF THE LAP LENGTH GIVEN ABOVE. ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

#### CONSTRUCTION REQUIREMENTS AND PROCEDURES:

THE CONTRACTOR SHALL TAKE THE PROPER PRECAUTIONS TO ENSURE THE STABILITY AND SAFE PERFORMANCE OF ALL STRUCTURAL ELEMENTS DURING DEMOLITION AND CONSTRUCTION. REFER TO APPROVED DEMOLITION/ERECTION PLANS.

IF THERE ARE REVISIONS TO APPROVED PLANS, THE CONTRACTOR SHALL SUBMIT THESE CHANGES TO THE ENGINEER OF RECORD AND MASSDOT FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

CONTRACTOR SHALL DISPOSE OF ANY DEMOLITION DEBRIS, CONSTRUCTION DEBRIS, WOOD WASTES, CONTAMINATED SOILS, HAZARDOUS MATERIALS AND OTHER SPECIAL WASTES IN STRICT ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS.

#### **EXISTING CONDITIONS:**

THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENTS AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF, AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL HE HAS MADE THE REQUIRED MEASUREMENTS AND THE EXTENT OF THE PROPOSED WORK HAS BEEN APPROVED BY THE ENGINEER.

#### **UTILITIES:**

THE CONTRACTOR SHALL LOCATE AND PROTECT FROM DAMAGE ALL EXISTING UTILITIES.

#### CONSTRUCTION JOINTS:

CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT THE APPROVAL OF THE ENGINEER.

#### SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS.

#### CONCRETE:

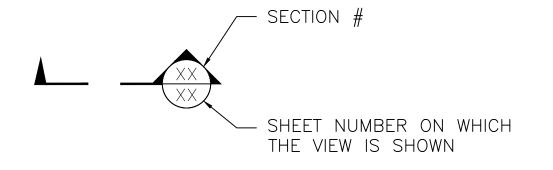
PRECAST STRIP FOOTINGS, WINGWALLS, WINGWALL FOOTINGS, APPROACH SLABS AND CAST—IN—PLACE CONCRETE SHALL BE 4000 PSI, \( \frac{3}{4} \) IN, 610 CEMENT CONCRETE.

PRECAST RIGID FRAME SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.

#### LOAD RATINGS:

FABRICATOR OR PRECAST RIGID FRAME MANUFACTURER SHALL SUBMIT LOAD RATING IN ACCORDANCE WITH THE 2013 MASSACHUSETTS DEPARTMENT OF TRANSPORTATION LRFD BRIDGE MANUAL.

#### SECTION MARK:



#### BUCKLAND NILMAN ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	NFA	2	11
	PROJECT FILE NO.	N/A	

**GENERAL NOTES** 

#### HYDRAULIC DESIGN DATA

DRAINAGE AREA:	1.41	SQUARE MILES
DESIGN FLOOD DISCHARGE:	212	CUBIC FEET PER SECOND
DESIGN FLOOD FREQUENCY:	10	YEARS
DESIGN FLOOD VELOCITY:	7.4	FEET PER SECOND
DESIGN FLOOD ELEVATION:	845.5	FEET, NAVD

#### BASE (100-YEAR) FLOOD DATA

BASE FLOOD DISCHARGE:	446	CUBIC FEET PER SECOND
BASE FLOOD ELEVATION:	846.8	FEET, NAVD

#### DESIGN AND CHECK SCOUR DATA

DESIGN SCOUR FLOOD EVENT RETURN FREQUENCY: 25 YEARS CHECK SCOUR FLOOD EVENT RETURN FREQUENCY: 50 YEARS

#### FLOOD OF RECORD

DISCHARGE:	UNKNOWN	CUBIC FEET PER SECOND
FREQUENCY (IF KNOWN):	100	YEARS
MAXIMUM ELEVATION:	UNKNOWN	FEET, NAVD
DATE:	AUGUST 1955	MONTH, YEAR

HISTORY OF ICE FLOES: NO EVIDENCE OF SCOUR AND EROSION: YES, COLLAPSED WINGWALL

#### **ESTIMATED QUANTITIES**

(NOT GUARANTEED)

DESCRIPTION	<u>QUANTITY</u>	<u>UNIT</u>
DEMOLITION OF BRIDGE NO. $B-28-017$	1	LS
BRIDGE EXCAVATION	600	CY
CLASS B ROCK EXCAVATION	8	CY
GRAVEL BORROW FOR BRIDGE FOUNDATIONS	50	CY
GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES	250	CY
CONTROLLED DENSITY FILL - NON-EXCAVATABLE	3	CY
TEMPORARY SHORING	1	LS
CONTROL OF WATER, STRUCTURE NO. B-28-017	1	LS
BRIDGE STRUCTURE, BRIDGE NO. B-28-017	1	LS

COMMONWEALTH OF MASSACHUSETTS
MassDOT, Highway Division
CONCEPTUAL DESIGN IS ACCEPTABLE
TO MASSDOT FOR CONTRACTING

STATE BRIDGE ENGINEER
DATE

24-August-2020	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE	ONLY PRINTS OF LATEST DATE

EL. 838.00

Boring No.: **B-1/1A** 

STATION: 7+00 OFFSET: 12.0' LEFT GROUND ELEVATION: 852' N: 3042903.93 E: 315473.00

BORING B-2

**BORING LOG** Boring No.: **B-1/1A** Project Name Page No.: 1 of 2 GEODESIGN INCORPORATED Nilman Road over Clark Brook Geotechnical / Construction / Environmental Engineers and Scientists
40 Farrell Street File No.: 837-92 P.O. Box 699 Windsor, VT 05089 Phone: 802-674-2033/Fax: 802-674-5943 Buckland, MA S. Burlington, VT 05403 Phone: 802-652-5140 Checked By: \_\_\_JFW\_ Boring Company: QC/QA Laboratories, Inc. Flush SS Date John Leonhardt GeoDesign Rep.: 4.0 in. 1.38 in. Dan Howey Date Started: December 10, 2018 Date Finished: December 11, 2018 Hammer Wt.: 140 lbs 140 lbs 12/10/18, 15:00 8.0 843.0 Wet sample, (Remark 2). . Coordinate: Hammer Fall: 30 in. ■ 12/11/18, 16:00 6.0 845.0 Open hole (after 5 hrs). Ground Surface Elevation (feet): Rig Type: CME 550X ATV 

▼ Sample Information Strata Sample Description Description Blows / 6 inch Interval S1 SS 24 8 0 11 5 4 3 S1A (Top 3"): Ground Asphalt. Silty Gravelly Sand S1B (Bottom 5"): Brown fine to coarse SAND, with Cobbles & Boulders (Possible Fill) some fine to coarse Gravel, little Silt, moist. S2) Medium dense, white/brown fine to coarse Gravel (possible quartz cobble fragments), some fine to coarse Sand, little Silt, moist. S3) Loose, orange brown fine to medium SAND, EL. 846.0 S3 SS 24 10 4 11 4 4 5 little Silt, moist/wet. S4) Very dense; | S4 | SS | 24 | 16 | 6 | 7 | 22 | 35 | 22 | S4A (Top 8"): Brown fine to medium SAND, some Silt. moist/wet. S4B (Bottom 8"): Orange brown, fine to coarse GRAVEL and fine to coarse SAND, trace Silt, S5) Dense, gray fine to coarse SAND and SILT, | S5 | SS | 24 | 13 | 9 | 11 | 14 | 26 | 38 trace fine Gravel, moist/wet. (With Inferred Cobbles & Boulders) S6) Very dense, similar to S5. BOTTOM OF FOOTING S7A (Top 10"): Similar to S-5 with faint layering. S7B (Bottom 5"): Silvery dark gray fine to coarse GRAVEL and fine to coarse SAND, trace Silt. (Possible weathered cobble or boulder.) S8) Very dense, similar to S5, except trace fine to S8 SS 24 10 19 33 23 29 32 Glacial Till (With Inferred Cobbles & Boulders) (Continued) S9) Very dense, similar to S5. 
 S9
 SS
 24
 13
 24
 25
 27
 24
 31
 of Exploration at 26.0 ft 1) Ground surface elevation shown is based on taped measurements made in the field by GeoDesign and a topographic plan provided by VHB and should be considered approximate. 2) Moisture descriptions and water levels noted below 4 feet deep may be affected by wash rotary drilling methods. 3) Initially used hollow stem augers (HSA) to start the borehole. HSA refusal at ~3' deep in B-1. Moved ~4' south to B-1A and encountered HSA refusal at ~ 2 feet deep. Switched to spin & wash drilling methods with flush joint casing. Advanced the boring open hole with the roller bit below 9' | deep. | 1) Stratification Lines Represent Approximate Boundary Between Material Types, Transitions May Be Gradual. | 1) Water Level Readings Have Been Made At Times And Under Conditions Stated, Fluctuations Of Groundwater May Occur Due To Other Factors Than Those Present At The Time Measurements Were Made. | 2) Water Level Readings Have Been Made At Times And Under Conditions Stated, Fluctuations Of Groundwater May Occur Due To Other Factors Than Those Present At The Time Measurements Were Made. A.C. = After coring; N.R. = Not Recorded.

3) Sample Type Coding: A=Auger; C=Core; D=Driven; G=Grab; PS=Piston Sampler; SS=Split Barrel (Split Spoon); ST=Shelby Tube; Geo=Direct Push GeoProbe V=Vane; WOR/H=Weight of Rod/Hammer
4) Proportions Used: Trace = 1-10%; Little = 10-20%; Some = 20-35%; And = 35-50%

BORING B-1/1A

STATION: 6+40

OFFSET: 9.5' LEFT

GROUND ELEVATION: 851'

N: 3042841.17

E: 315449.35

**BORING LOG** Boring No.: B-2 Project Name Page No.: 1 of 2 GEODESIG N Nilman Road over Clark Brook File No.: 837-92 Geotechnical / Construction / Environmental Engineers and Scientists P.O. Box 699 40 Farrell Street Buckland, MA Checked By: \_\_\_JFW\_\_ Windsor, VT 05089 Phone: 802-674-2033/Fax: 802-674-5943 Casing: Sampler: QC/QA Laboratories, Inc. Groundwater Observations Flush SS Date Depth Elev. John Leonhardt GeoDesign Rep.: 4.0 in. 1.38 in. Dan Howey December 11, 2018 Date Finished: December 11, 2018 Hammer Wt.: 140 lbs ▼ 12/11/18, 12:00 2.0 850.0 Wet sample (perched). Date Started: N. Coordinate: Hammer Fall: 30 in. ■ 30 in. ■ 12/11/18, 12:30 6.0 846.0 Wet sample (Remark 2). E. Coordinate: Ground Surface Elevation (feet): Sample Information Sample Description Description S1) Medium dense, brown fine to medium SAND S1 SS 24 12 0 6 5 6 8 little Silt, little fine to coarse Gravel, moist. S2) Medium dense, brown fine to medium SAND, S2 SS 24 11 2 5 7 7 4 some Silt, trace fine to coarse Gravel, wet. S3) Medium dense, brown fine to medium SAND, S3 SS 24 11 4 4 8 7 7 little (-) Silt, trace fine Gravel, moist. S4) Medium dense, brown fine to coarse SAND, S4 SS 24 14 6 7 7 5 some Silt, trace fine Gravel, moist/wet. S5) Similar to S4, except some fine to coarse | S5 | SS | 24 | 8 | 9 | 8 | 7 | 8 | 10 | Silty Gravelly Sand S6) Similar to S4. | S6 | SS | 24 | 4 | 11 | 12 | 18 | 7 | 6 | S7) Loose, brown fine to coarse SAND, some Silt, 
 S7
 SS
 24
 7
 14
 4
 4
 5
 6
 little fine to coarse Gravel, moist/wet. S8) Very dense, brown fine to coarse SAND, | S8 | SS | 20 | 9 | 19 | 12 | 22 | 79 | 50/2" | some fine to coarse Gravel, little silt, moist/wet. S9) Very dense, brown fine to coarse SAND, little S9 SS 24 12 24 28 49 43 36 fine to coarse Gravel, little Silt, moist/wet. Inferred 823.0 S10) Refusal, gray fine to coarse SAND, some Weathered Bedrock Silt, little fine to coarse Gravel (friable, inferred S10 SS 12 10 29 45 100 Silt, little fine to coarse Gravel (friable, inferred weathered bedrock), moist. S11) Similar to S10. S11 SS 9 7 34 78 60/3" of Exploration at 35.3 ft 1) Ground surface elevation shown is based on taped measurements made in the field by GeoDesign and a topographic plan provided by VHB and 2) Moisture descriptions and water levels noted below 4 feet deep may be affected by wash rotary drilling methods. 3) Borehole advanced by drive and wash methods to 29 feet deep. Boring was advanced open hole with the roller bit below 29 feet deep. 4) After obtaining sample S-8 and advancing ahead to 24 feet deep with roller bit, water flow (estimated ~ 2 gpm) was observed from casing approximately 1 foot above ground surface indicating slight artesian condition. The flow was observed intermittently throughout the remainder of 5) Per prior agreement, the boring was backfilled by the Buckland Highway Department with granular backfill with a plan to check the seepage condition at a later date. The boring was checked by GeoDesign on 12/15/18 and was observed to still be seeping water. 1) Stratification Lines Represent Approximate Boundary Between Material Types, Transitions May Be Gradual. 1) Stratification Lines Represent Approximate Boundary Between Material Types, I ransitions May be Gradual.

2) Water Level Readings Have Been Made At Times And Under Conditions Stated, Fluctuations Of Groundwater May Occur Due To Other Factors Than Those Present At The Time Measurements Were Made.

A.C. — After coring, N.R. — Not Recorded.

3) Sample Type Coding, A—Auger, C—Core; D—Driven; G—Grab; PS=Piston Sampler; SS—Split Barrel (Split Spoon); ST=Shelby Tube; Geo= Direct Push GeoProbe V=Vane;

WOR.H—Weight of Rod/Hammer

4) Proportions Used: Trace = 1-10%; Little = 10-20%; Some = 20-35%; And = 35-50%

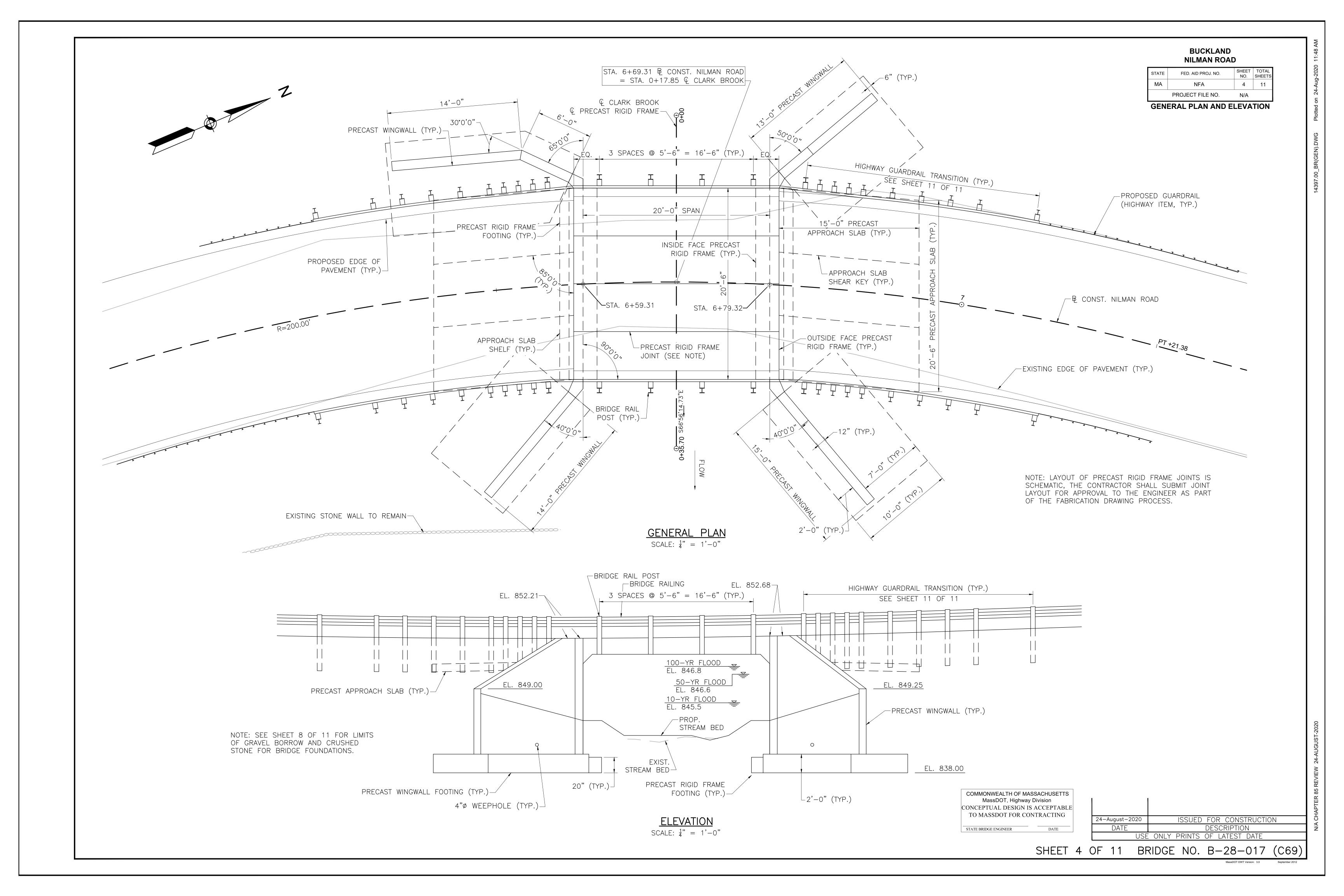
Boring No.: **B-2** 

NOTES:

- 1. LOCATION OF BORINGS FOR PROPOSED BRIDGE SHOWN ON THE KEY PLAN THUS: ● B-1
- 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. WATER LEVELS SHOWN ON THE BORING LOGS AND INDICATED THUS: \( \bigsize \) WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 1%" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30", UNLESS OTHERWISE NOTED.
- 5. ALL BORINGS FOR PROPOSED BRIDGE WERE MADE IN DECEMBER 2018.
- 6. BORINGS WERE MADE BY QC/QA LABORATORIES, INC.
- 7. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division CONCEPTUAL DESIGN IS ACCEPTABLE TO MASSDOT FOR CONTRACTING STATE BRIDGE ENGINEER

ISSUED FOR CONSTRUCTION 24-August-2020 DESCRIPTION DATE USE ONLY PRINTS OF LATEST DATE



**BUCKLAND NILMAN ROAD** FED. AID PROJ. NO. 5 11 PROJECT FILE NO. LONGITUDINAL SECTION 11 MIN. SUPERPAVE BRIDGE SURFACE COURSE OVER  $1\frac{1}{2}$ " SUPERPAVE BRIDGE PROTECTIVE COURSE OVER SPRAY APPLIED MEMBRANE WATERPROOFING 20'-6" 艮 CONST. BRIDGE RAIL (TYP.) EL. 852.21 (SOUTH)
EL. 852.68 (NORTH) VARIES (10'-0" MIN.) VARIES (10'-0" MIN.) EL. 852.41 (SOUTH) EL. 852.66 (NORTH)— \_VARIES (3" MIN.) PRECAST WINGWALL (TYP.) 2.0% 2.0% TOP ROOF EL. 851.95 PROP. PRECAST RIGID FRAME ROOF -HAUNCH\_ EXPANSION JOINT (TYP.)— APPROACH SLAB SHELF PROP. 2:1 SLOPE PROP. RIGID ∕—EL. 842.55 FRAME JOINT (TYP.) EL. 842.00 PROP. STREAM BED FLOW 4"ø WEEPHOLE (TYP.)  $\bigcirc$ SEE NOTE 2 LEXIST. STREAM BED EL. 838.00 PRECAST WINGWALL FOOTING (TYP.) PRECAST RIGID FRAME STRIP FOOTING LONGITUDINAL SECTION NOTES: SCALE:  $\frac{1}{2}$ " = 1'-0" 1. 'A' = PRECAST RIGID FRAME ROOF THICKNESS TO BE DETERMINED BY FABRICATOR. MINIMUM THICKNESS = 12", MAXIMUM THICKNESS = 15". 2. 4" Ø WEEP HOLES 10'-0" O.C. PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE. COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division CONCEPTUAL DESIGN IS ACCEPTABLE TO MASSDOT FOR CONTRACTING ISSUED FOR CONSTRUCTION

SHEET 5 OF 11 BRIDGE NO. B-28-017 (C69)

24-August-2020

DATE

DATE

STATE BRIDGE ENGINEER

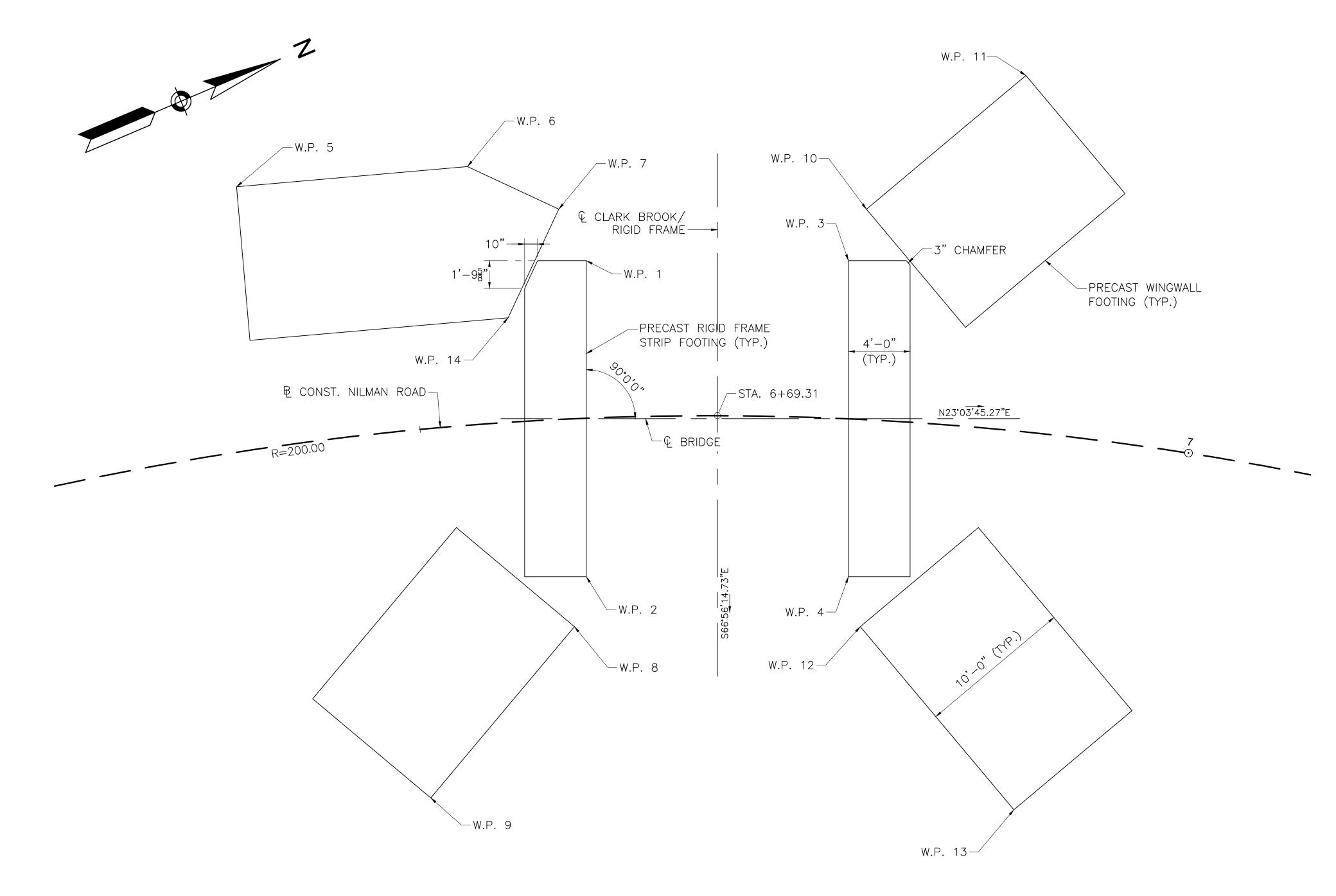
DESCRIPTION

USE ONLY PRINTS OF LATEST DATE



STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	NFA	6	11
	PROJECT FILE NO.	N/A	

FOUNDATION PLAN



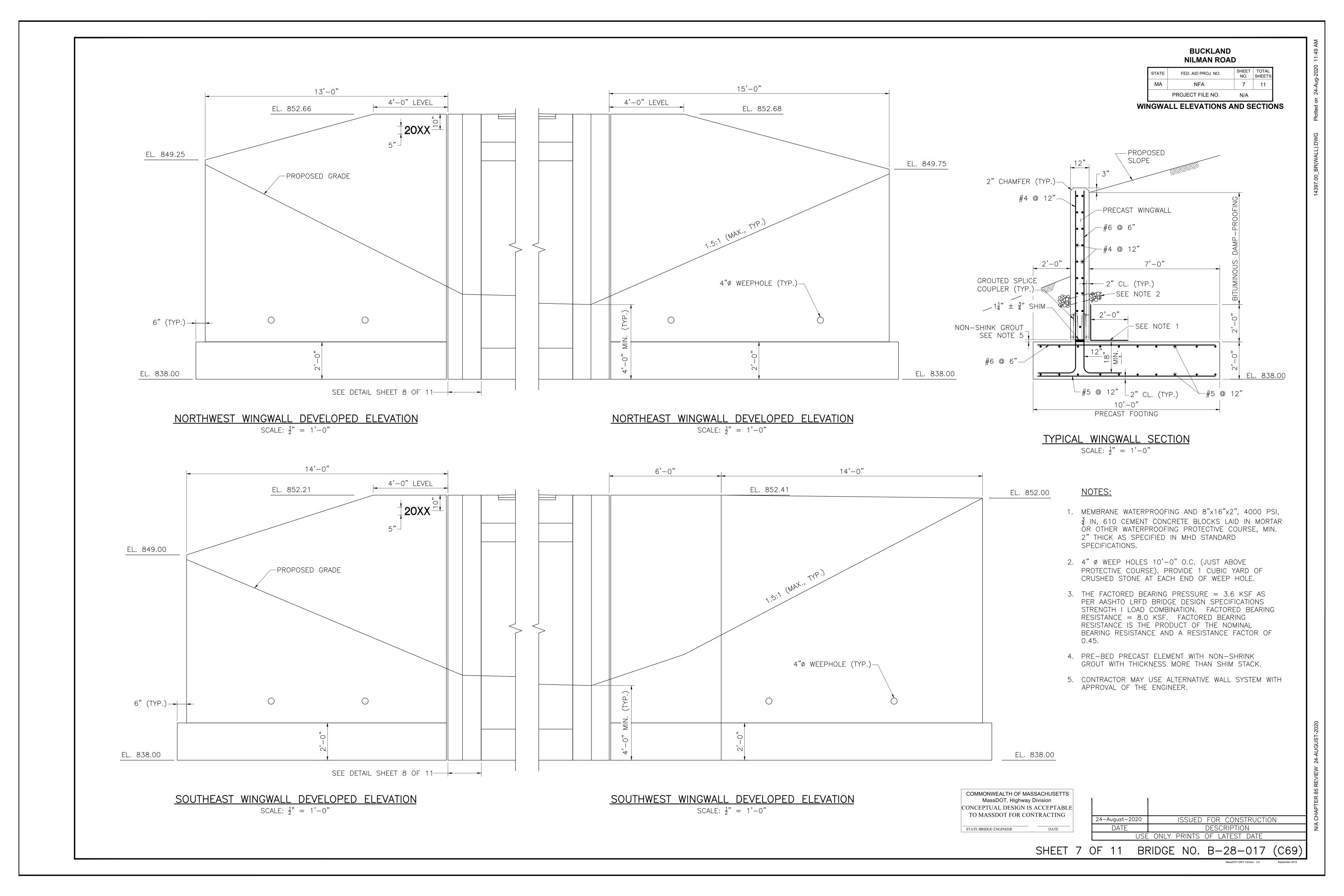
	W	ORKING POINT	TS .	
W.P. #	STA.	OFFSET (FT)	NORTHING	EASTING
1	6+61.20	10.21 LT	3042865.505	315455.761
2	6+60.38	10.28 RT	3042857.474	315474.623
3	6+77.38	10.25 LT	3042881.146	315462.421
4	6+78.30	10.23 RT	3042873.116	315481.282
5	6+40.44	17.03 LT	3042846.506	315442.463
6	6+54.30	16.73 LT	3042860.801	315447.125
7	6+59.65	13.62 LT	3042865.169	315451.987
8	6+59.40	13.48 RT	3042855.494	315477.305
9	6+48.23	23.86 RT	3042842.567	315483.873
10	6+78.34	13.63 LT	3042883.531	315459.811
11	6+87.23	23.01 LT	3042896.446	315455.878
12	6+79.29	13.43 RT	3042872.555	315484.568
13	6+91.34	24.45 RT	3042877.071	315499.396
14	6+56.16	6.77 LT	3042859.386	315457.182

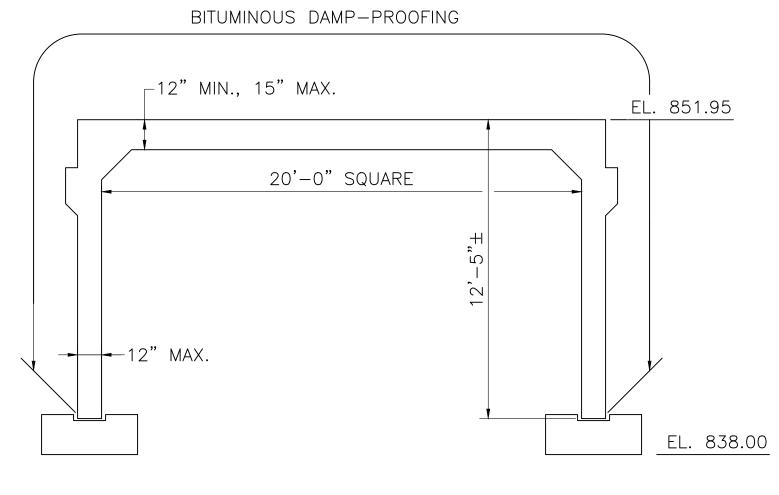
FOUNDATION PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

COMMONWEALTH OF MASSACHUSETTS
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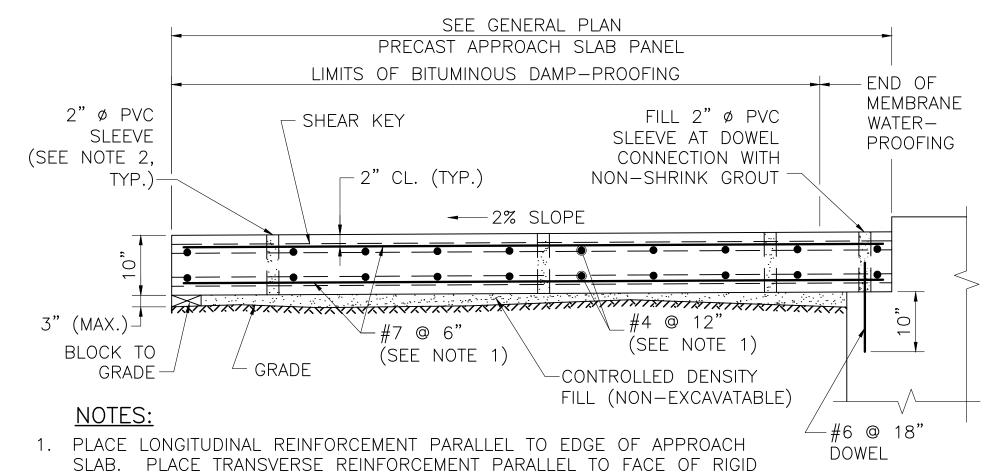




- 1. 'A' TO BE DETERMINED BY FABRICATOR. MINIMUM = 12", MAXIMUM = 15"
- 2. SEE APPROACH SLAB DETAIL FOR LIMITS OF WATERPROOFING.

NOTES:

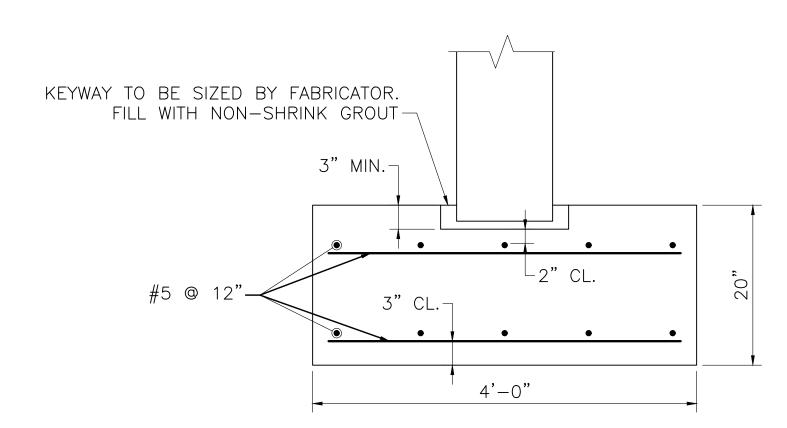
# CULVERT TYPICAL SECTION SCALE: $\frac{1}{4}$ " = 1'-0"



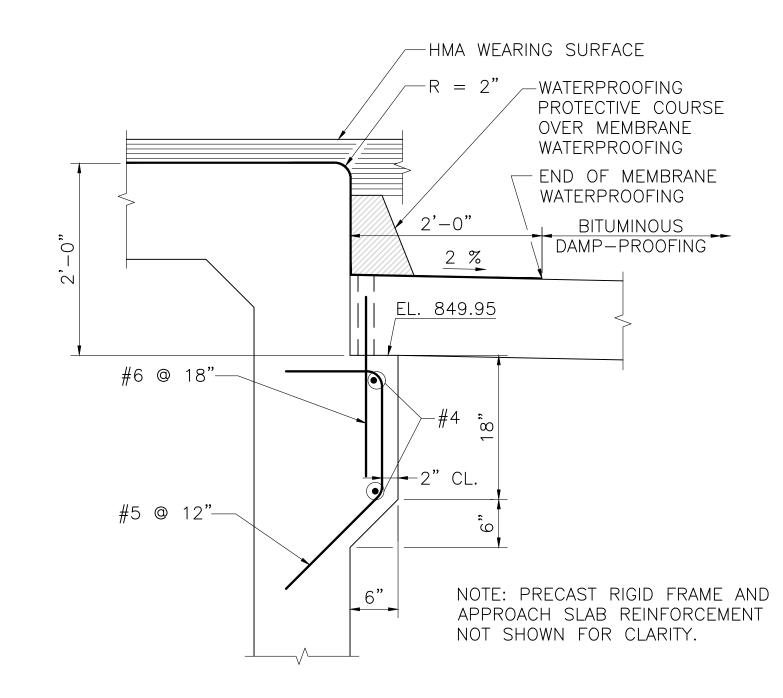
FRAME. 2. PVC SLEEVES TO BE INCLUDED IN PRECAST APPROACH SLABS TO FACILITATE

PLACEMENT OF CONTROLLED DENSITY FILL (NON-EXCAVATABLE).

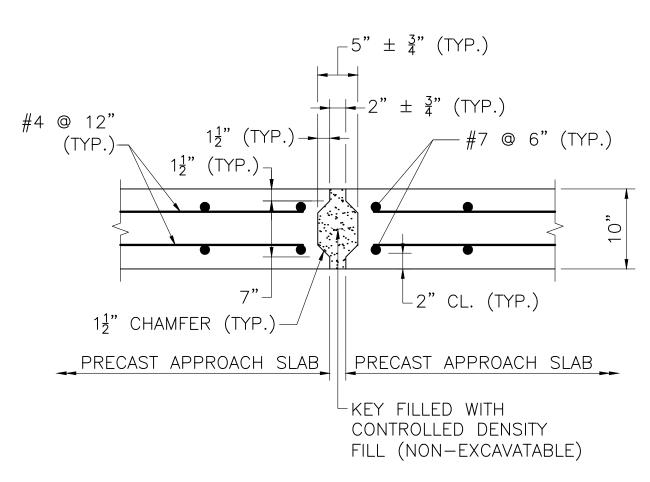
# PRECAST APPROACH SLAB SCALE: $\frac{3}{4}$ " = 1'-0"



RIGID FRAME STRIP FOOTING DETAIL SCALE: 1" = 1'-0"



## PRECAST APPROACH SLAB SHELF - DETAILS SCALE: 1" = 1'-0"



### APPROACH SLAB SHEAR KEY DETAIL SCALE: $\frac{3}{4}$ " = 1'-0"

# NOTES:

- 1. THE FACTORED BEARING PRESSURE = 6.4KSF AS PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS STRENGTH I LOAD COMBINATION.
- 2. FACTORED BEARING RESISTANCE = 7.0 KSF. FACTORED BEARING RESISTANCE IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE AND A RESISTANCE FACTOR OF 0.45.

# 6" MIN.-

-GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES-CRUSHED STONE FOR BRIDGE FOUNDATIONS-6" MIN.--BOTTOM OF EXCAVATION OF UNSUITABLE MATERIAL. FINAL ELEVATION TO BE DETERMINED  $\phi = 45^{\circ}$  FOR DEPTH 5'-0" OR LESS BY RESIDENT ENGINEER.  $\phi = 60^{\circ}$  FOR DEPTH OVER 5'-0"

NOTE: CRUSHED STONE FOR BRIDGE FOUNDATIONS SHALL BE ENVELOPED IN NON-WOVEN GEOTEXTILE FABRIC.

PRECAST RIGID FRAME

PRECAST WINGWALL

FACE OF

CULVERT

**BUCKLAND NILMAN ROAD** 

**MISCELLANEOUS DETAILS** 

8 11

←CAST-IN-PLACE

-1" PREFORMED

FILLER

CONCRETE

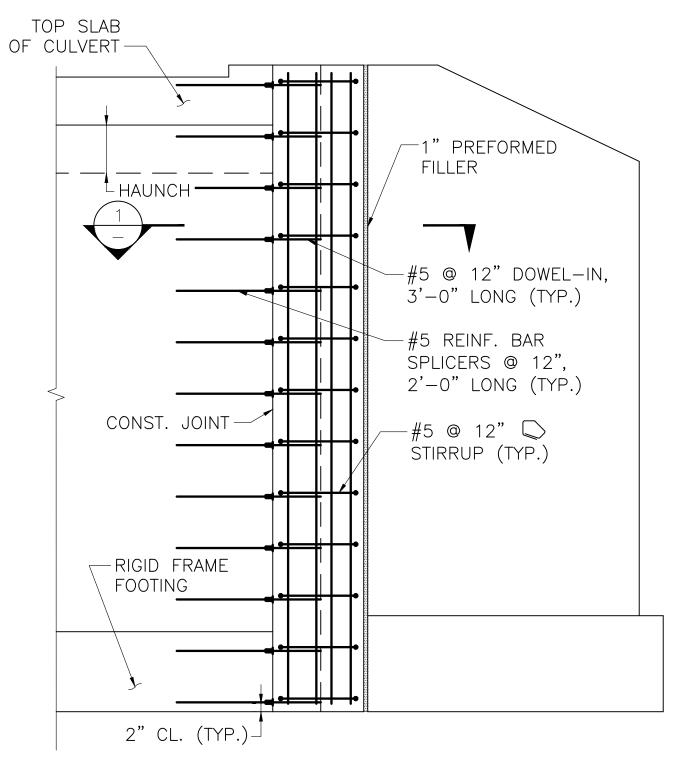
FOOTING —

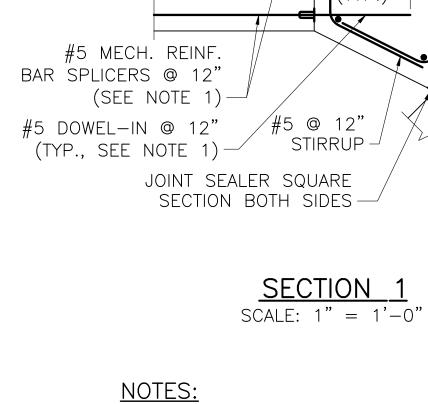
WINGWALL

FED. AID PROJ. NO.

PROJECT FILE NO.

#### LIMITS OF GRAVEL BORROW AND CRUSHED STONE NOT TO SCALE





#6 (TYP.)-

- 1. THE CONTRACTOR MAY SUBSTITUTE #5 DOWELS, 3'-0" LONG, FOR MECHANICAL REINFORCING BAR SPLICERS AND THREADED REBARS.
- 2. CULVERT AND WINGWALL REINFORCEMENT IS NOT SHOWN FOR CLARITY.

SECTION AT WINGWALL SCALE:  $\frac{1}{2}$ " = 1'-0"

COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division CONCEPTUAL DESIGN IS ACCEPTABLE TO MASSDOT FOR CONTRACTING DATE STATE BRIDGE ENGINEER

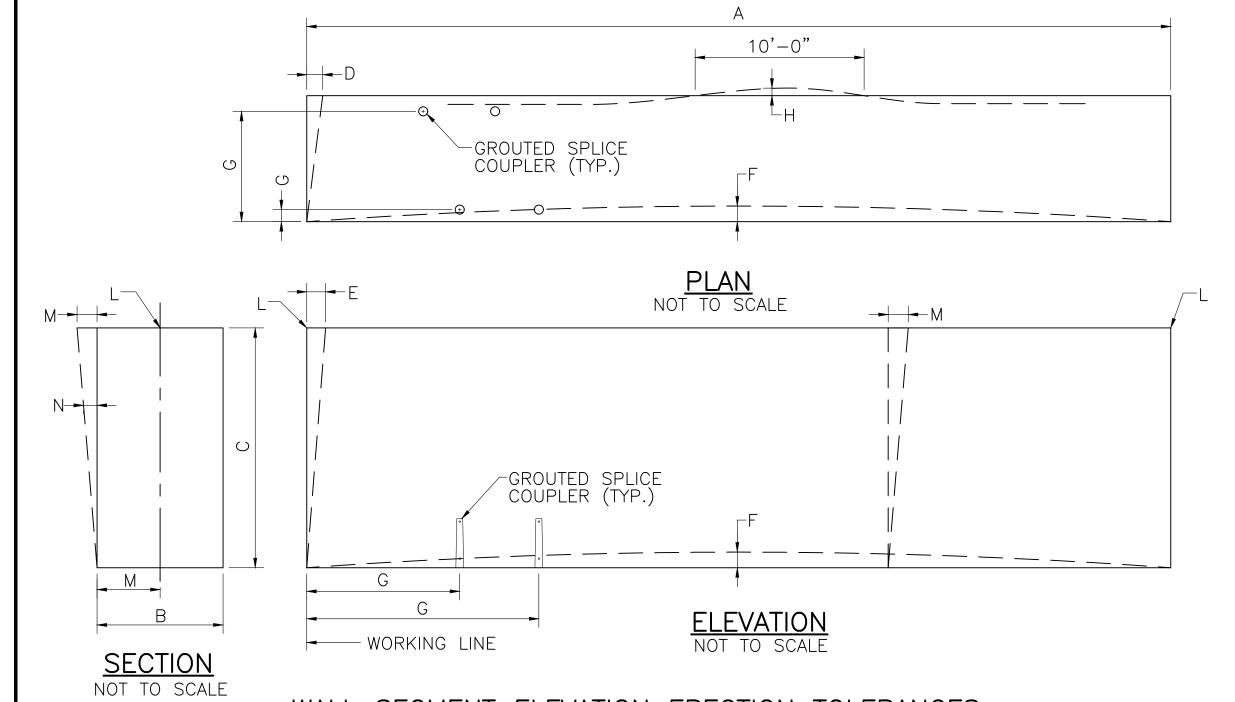
24-August-2020	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USF	ONLY PRINTS OF LATEST DATE

SHEET 8 OF 11 BRIDGE NO. B-28-017 (C69)

#### BUCKLAND NILMAN ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	NFA	9	11
	PROJECT FILE NO.	N/A	

PRECAST DETAILS AND TOLERANCES

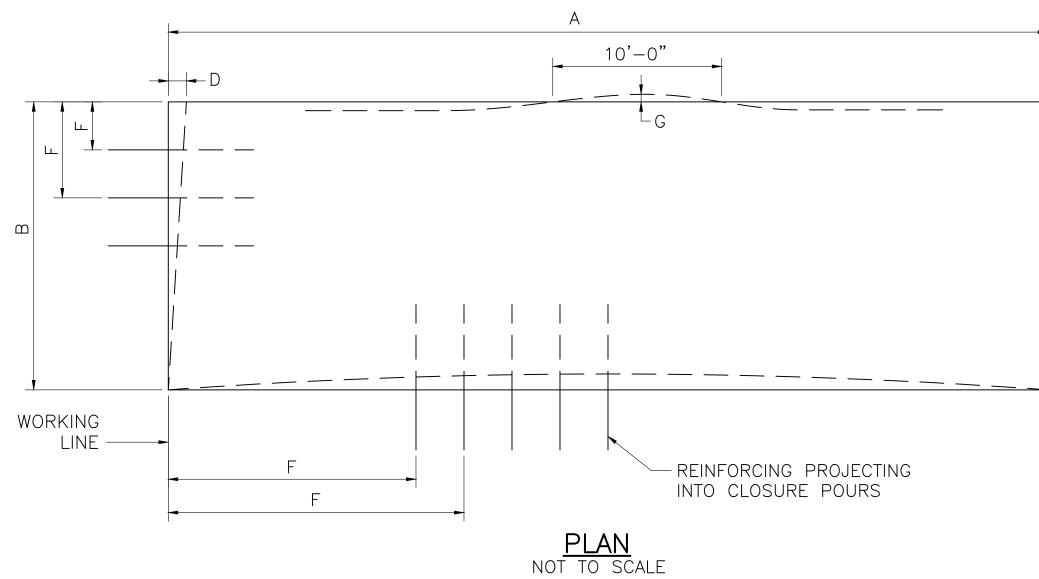


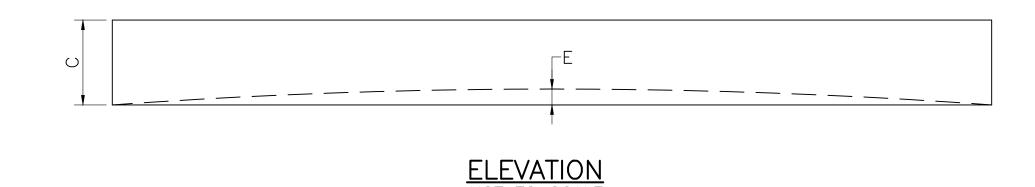
# WALL SEGMENT ELEVATION ERECTION TOLERANCES

L	TOP ELEVATION FROM NOMINAL TOP ELEVATION	1"
М	MAXIMUM PLUMB VARIATION OVER HEIGHT OF PANEL	<u>1</u> "
N	PLUMB IN ANY 10 FEET OF PANEL HEIGHT	<u>1</u> "

# WALL SEGMENT FABRICATION TOLERANCES

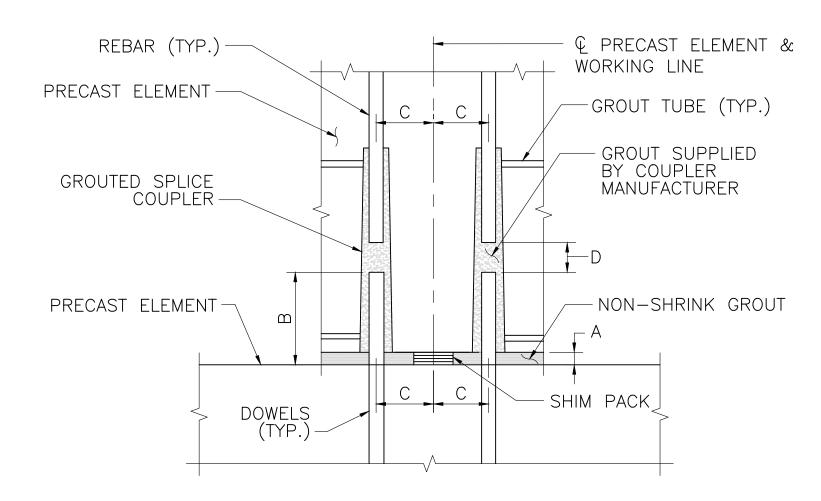
Α	LENGTH	± <sup>1</sup> / <sub>4</sub> "
В	WIDTH (OVERALL)	土11"
С	DEPTH (OVERALL)	± <sup>1</sup> / <sub>4</sub> "
D	VARIATION FROM SPECIFIED PLAN END SQUARENESS OR SKEW	± <sup>1</sup> / <sub>2</sub> "
Е	VARIATION FROM SPECIFIED ELEVATION END SQUARENESS OR SKEW	± <sup>1</sup> / <sub>2</sub> "
F	SWEEP OVER MEMBER LENGTH	±3"
G	LOCATION OF GROUTED SPLICE COUPLER MEASURED FROM A WORKING LINE	± <sup>1</sup> / <sub>4</sub> "
Н	LOCAL SMOOTHNESS OF ANY SURFACE	$\pm \frac{1}{4}$ " IN 10 FEET





# APPROACH SLAB FABRICATION TOLERANCES

А	LENGTH (OVERALL)	土11"
В	WIDTH (OVERALL)	土1"
С	DEPTH (OVERALL)	± <sub>4</sub> "
D	VARIATION FROM SPECIFIED PLAN END SQUARENESS OR SKEW	± <sub>2</sub> "
E	SWEEP OVER MEMBER LENGTH	士8"
F	LOCATION OF PROJECTING REINFORCING MEASURED FROM A WORKING LINE	±½"
G	LOCAL SMOOTHNESS OF ANY SURFACE	$\pm \frac{1}{4}$ " IN 10 FEET



#### NOTES

- 1. USE MATCHING TEMPLATES FOR THE LOCATION OF REINFORCEMENT AND GROUTED SPLICE COUPLER PLACEMENT WITHIN THE ELEMENTS TO CONTROL THE CRITICAL DIMENSION "C".
- 2. CONSULT MANUFACTURER OF THE GROUTED SPLICE COUPLER FOR PROPER DIMENSIONS "B" AND "D" AND FOR TOLERANCES ON THESE AND ALL DIMENSIONS.
- 3. BEFORE EXECUTING GROUTED SPLICE COUPLER ASSEMBLIES, ALWAYS SEEK INSTALLATION RECOMMENDATIONS FROM THE MANUFACTURER OF THE GROUTED SPLICE COUPLER USED.

# GROUTED SPLICE COUPLER DETAILS NOT TO SCALE

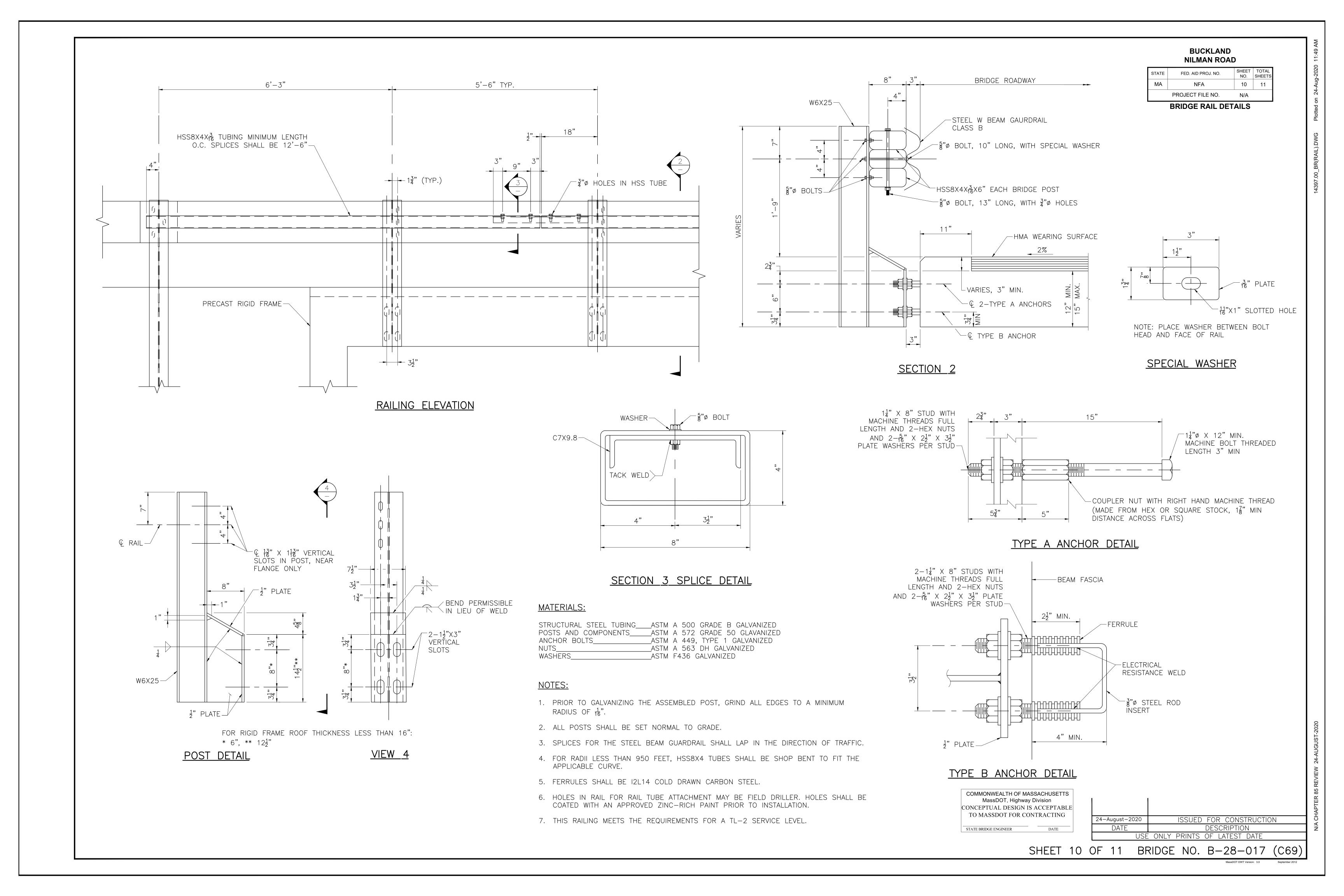
# GROUTED SPLICE COUPLER TOLERANCES

Α	SHIM PACK HEIGHT	$1\frac{1}{4}$ " $\pm \frac{3}{4}$ "
В	DOWEL HEIGHT	CONSULT MANUFACTURER
С	LOCATION OF REINFORCING, GROUTED SPLICE COUPLER, AND DOWELS MEASURED FROM A WORKING LINE	土 1 "
D	GAP BETWEEN DOWELS AND REINFORCING	CONSULT MANUFACTURER

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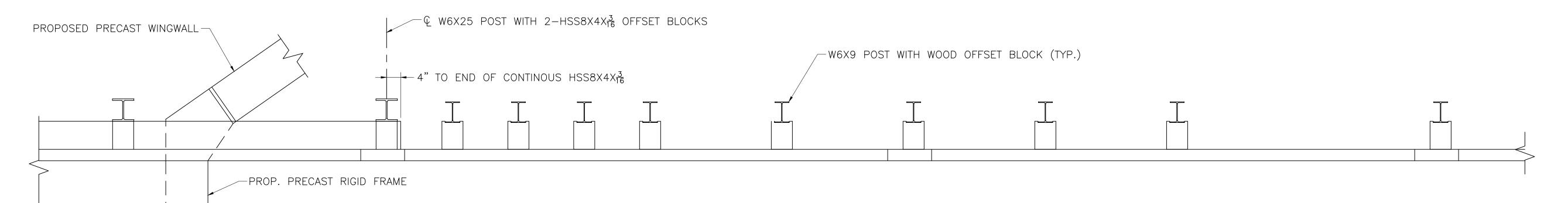
STATE BRIDGE ENGINEER
DATE

24-August-2020	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE	ONLY PRINTS OF LATEST DATE

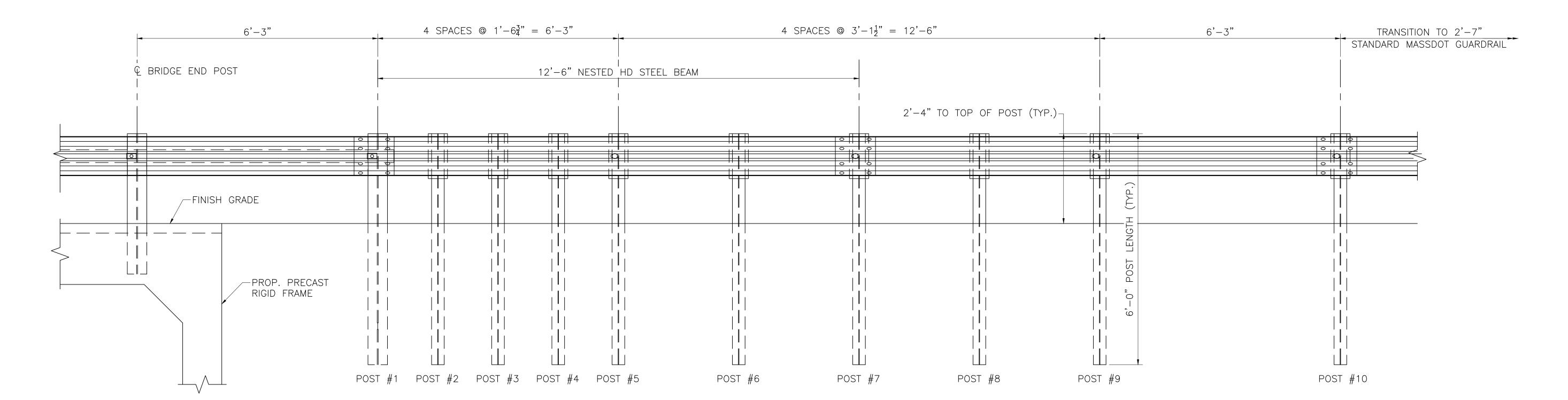


TE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
Α	NFA	11	11
	PROJECT FILE NO.	N/A	

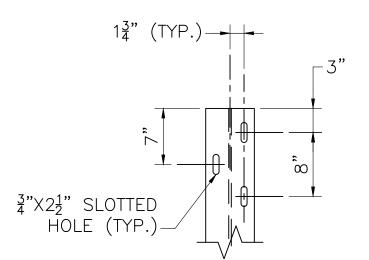
HIGHWAY GUARDRAIL TRANSITION



# RAILING TRANSITION PLAN



# RAILING TRANSITION ELEVATION



POST 1 HOLE DETAIL

# NOTES:

- 1. OFFSET BLOCKS SHALL BE RECESSED WOOD ONLY. STEEL OR PLASTIC OFFSET BLOCKS ARE NOT PERMITTED.
- 2. GUARDRAIL IS NOT ATTACHED TO POST NUMBERS 2-4, 6 AND 8. THERE SHALL BE NO GAP BETWEEN THE POSTS THAT ARE NOT ATTACHED TO THE RAIL. OFFSET BLOCKS SHALL BE ATTACHED TO POST WITH STANDARD POST BOLT.
- 3. POST MAY BE SET IN DRILLED HOLES OR DRIVEN TO GRADE.
- 4. THIS RAILING MEETS THE REQUIREMENTS FOR A NCHRP REPORT 350 TL-3 SERVICE LEVEL.

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